

FF 100 PANEL



UL LISTED CONSTRUCTION NUMBERS

#529 SECTION ANALYSIS REPORT AND SPAN LOAD TABLES

16265 E 33rd Dr Suite 40, Aurora, CO 80011

PH 303-294-0538 **** 800-574-1717 **** FAX 303-294-9407

File R14692 Vol. 1 Sec. 3 Page 1 Issued: 2003-11-19

DESCRIPTION

PRODUCT COVERED:

This section of the Procedure covers a coated steel roof panel, which is identified as "FF100" panel. The panel is produced at job sites by portable rolling machines.

The panel is roll-formed from No.29 MSG minimum or heavier gauge steel coated to the configuration shown in Ill. 1. The panel may also have a paint finish over the coating.

SPECIFICATIONS OF FINISHED PRODUCT:

THICKNESS

The base metal thickness of the steel used in the fabrication of the panel shall be not less than 0.0128 in. No. 29 msg minimum gauge. This thickness shall not include any coating or paint finish.

DIMENSIONS

The cross-sectional dimensions of the panel piece shall be in accordance with the cross-section in Ill. 1.

STRENGTH

The strength records of the steel shall be reviewed. The 29 MSG steel used shall conform to ASTM A792 grade 50 Specifications or the minimum yield point of the steel shall be 50,000 psi.



Northbrook Division

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NEW TECH MACHINERY CORP MR G BATTISTELL 1300 40TH ST DENVER CO 80205

RE: Project Number(s) - 03NK22866

Your most recent Certification is shown below. You may also view this information, or a portion of this information (depending on the product category), on UL's Online Certifications Directory at www.ul.com/database. Please review the text and contact the Conformity Assessment Services staff member who handled your project if revisions are required. For instructions on placing an order for this information in a 3 x 5-inch format, you may refer to the enclosed order form for UL Card Service.

TIPV Metal Roof Deck Panels November 21, 2003

NEW TECH MACHINERY CORP

R14692

1300 40TH ST, DENVER CO 80205

Coated steel panels, field - formed.

Underwriters Laboratories Inc. Metal Roof Deck Panels, Fabricated, installed and used in the following roof deck constructions with corre-Underwriters Laboratories Inc. Metal Roof Deck Panels, Fabricated, installed and used in the following roof dec sponding panel identifications:

Coated steel panels identified as "Snap Panel 550" for use in Construction No. 373.

Coated steel panels identified as "Panel 210A" for use in Construction Nos. 90, 176, 180, 238, 238A.

Coated steel or aluminum panels identified as "Snap Panel 675" for use in Construction Nos 254, 255, 261, 303.

Coated steel panels identified as "SS450" for use in Construction No. 370.

Coated steel panels identified as "SS150" for use in Construction No. 554.

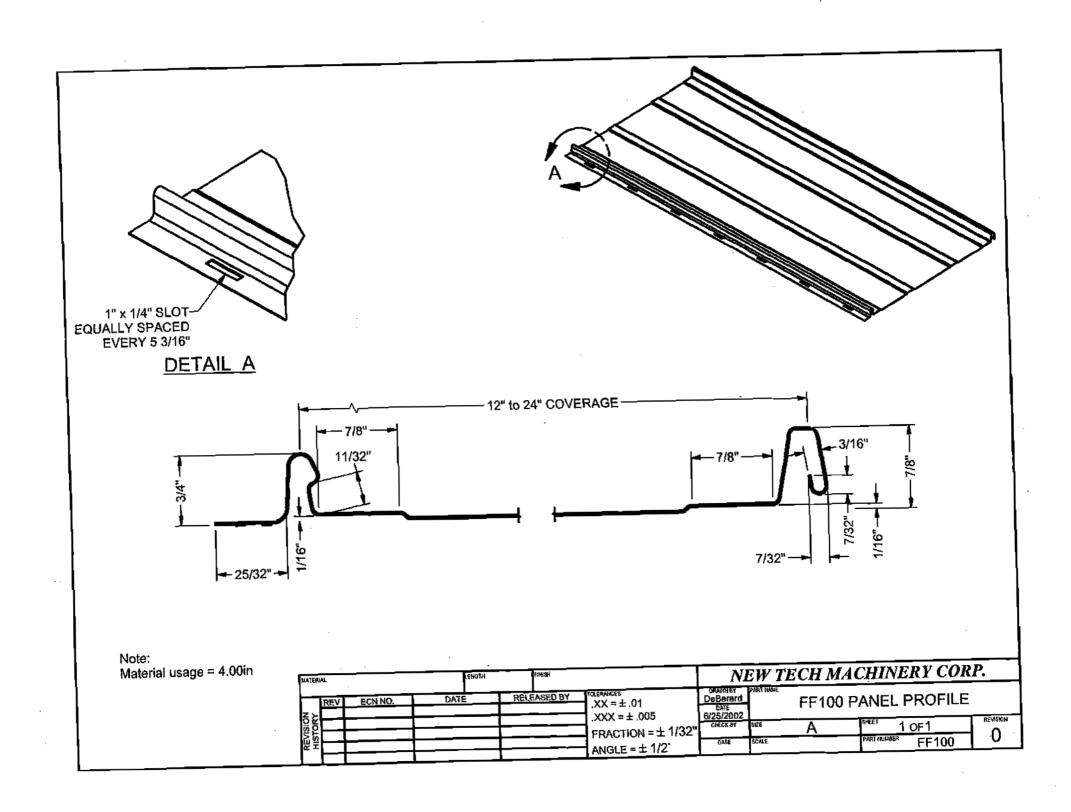
Coated steel panels identified as "SS150" for use in Construction No. 575.

Coated steel panels identified as "FF100" for use in Construction No. 579.

Coated steel panels identified as "FF100" for use in Construction No. 529.

See Roof Deck Construction for description of construction numbers.

LOOK FOR LISTING MARK ON PRODUCT





Online Certifications Directory

TGKX.529 Roof Deck Constructions

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Roof Deck Constructions

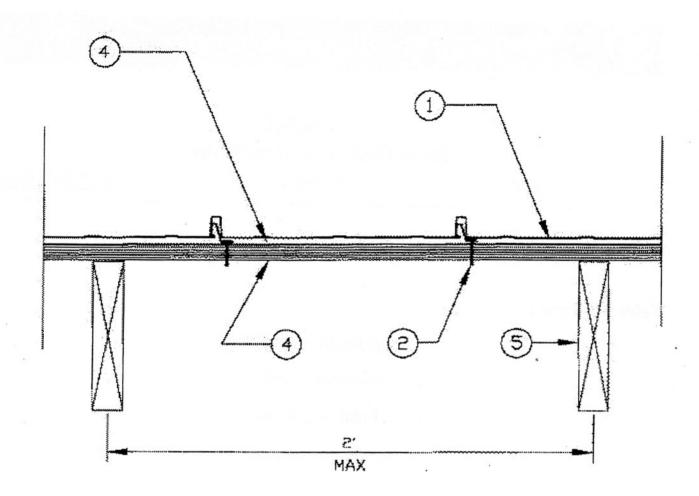
Guide Information

Construction No. 529

November 21, 2003

Uplift — Class 90

Fire Not Investigated



1. **Metal Roof Deck Panels*** — No. 29 MSG min coated steel. Max panel width 24 in. Rib height nominal 1 in. Panels continuous over two or more spans. A bead of sealant may be used at panel endlaps.

AMERICAN BUILDING COMPONENTS — "SL-16"

CENTRAL TEXAS METAL ROLLFORMING INC — "PRO-SNAP 100"

MBCI — "Slimline"

NCI BUILDING SYSTEMS L P — "Slimline"

NEW TECH MACHINERY CORP — "FF100"

UNION CORRUGATING CO — "Advantage Lok"

2. Fasteners — (Screws) — For panel attachment to wood deck (Item 3), fasteners to

be No. 10 x 1 in. long No. 2 Phillips, Pancake Head Type A. Fasteners spacing to be 12 in. OC with fasteners installed through prepunched slots in fastener flange of panel. For attachment of plywood deck (Item 3) to joists (Item 5), fasteners to be min. No. 6 by 1-7/8 bugle head screw or annular ring-shank nails. Spacing to be 6 in. OC at plywood edges and 12 in. OC at intermediate supports. When light gauge structural steel joists are used, fasteners to be No. 12 by 1-5/8 in. long with a Phillips head.

- 3. Substructure (Plywood) Plywood decking to be a nom 5/8 in. thick, exposure sheathing span C-D, 40/20 plywood. All butt joints to be sealed against leakage by using tape and/or caulk or with one-part urethane sealant.
- 4. **Moisture Barrier** (Optional) Any suitable membrane to protect substructure (Item 3).
- 5. Joists Joists spaced at 2 ft, 0 in. OC, may be one of the following:
 - A. Nom 2 by 6 in. wood joists No. 2 or better.
 - B. Nom 2 by 4 in. wood when used on a top cord of a wood truss, No. 2 or better.
 - C. Light gauge structural steel framing with the member against the plywood to be a min No. 22 MSG coated steel.

Refer to General Information, Roof Deck Construction, (Roofing Materials and Systems Directory) for Items Not Evaluated.

*Bearing the UL Classification Mark

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JOHN F. BUTTS & ASSOC., INC.

CONSULTING ENGINEERS
2480 VANTAGE DRIVE COLORADO SPRINGS, CO 80919
(719) 598-7666 FAX (719) 598-0258

www.ifba.com

August 29, 2002

New Tech Machinery Corp. 1300 40th Street Denver, CO 80205-3311

Re: Section Analysis Report New Tech FF100 Panel

Job No. 183-05

Gentlemen:

Per your request, please find enclosed the engineering calculations for the above referenced project. The section, with the structural properties indicated in this report is certified to meet or exceed the requirements of the 1996 AISI Cold-Formed Steel Design Manual including Supplement No. 1 (July 1999).

Please note that the panel analysis and Load Tables have been evaluated based on the assumption that the proper bearing, side laps, end laps, bracing, anchorage and structural supports are being utilized in the member's installation. We do not certify the installation method, attachment and supporting materials.

If we can be of further assistance or if you require additional information, please call.

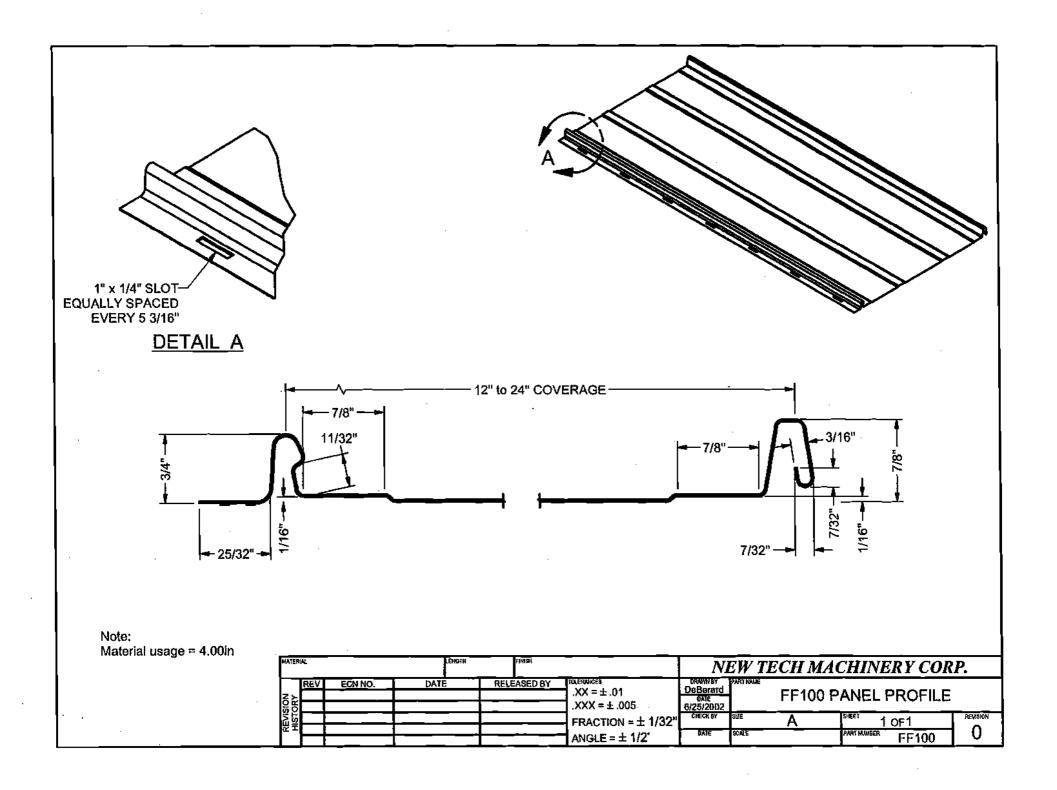
Sincerely,

John F. Butts, P.E.

President

enc. Section Drawing

Section Analysis Section Load Tables



John F. Butts & Associates, Inc. 2480 Vantage Drive Colorado Springs, CO 80919 (719) 598-7666

Analysis per 1996 AISI Cold-Formed Steel Manual + 1999 Supplement 1

New Tech FF100 Panel

FILE: FF100X12

SECTION DIMENSIONS

Line #1 Angle(L)	= -	80.000 deg	Line #21 Angle(R) =	81.000 deg
Line #1 Radius(L)	=	0.060 in	Line #21 Radius(R) =	0.063 in
Line #1 Length(L)	=	0.000 in	Line #22 Length(R) $=$	0.194 in
Line #2 Angle(L)	=	80.000 deg	Line #22 Angle(R) =	70.000 deg
Line #2 Radius(L)	=	0.070 in	Line #22 Radius(R) =	0.060 in
Line #2 Length(L)	=	0.683 in	Line #24 Length(R) =	0.034 in
Line #3 Angle(L)	=	81.000 deg	Line #23 Angle(R) =	-86.000 deg
Line #3 Radius(L)	=	0.070 in	Line #23 Radius(R) =	0.070 in
Line #3 Length(L)	=	0.000 in	Line $#26 \text{ Length}(R) =$	0.134 in
•		80.000 deg	Line #24 Angle(R) =	-151.000 deg
Line #4 Radius(L)	=	0.070 in	Line #24 Radius(R) =	0.122 in
Line #4 Length(L)		0.125 in	Line #28 Length(R) =	0.447 in
	=	0.000 deg	Line #25 Angle(R) $=$	86.000 deg
Line #5 Radius(L)	=	0.000 in	Line #25 Radius(R) =	0.141 in
Line #5 Length(L)		0.000 in	Line #30 Length(R) \Rightarrow	0.636 in
			5 . ,	

Panel Bottom Width = 12.00 in Panel Overall Width = 12.56 in Panel Overall Height = 0.81 in

Alloy: ASTM A792, G50

Fy = 50.00 ksiFy = 14.13 ksi

QUALIFICATIONS PER AISI SPECIFICATIONS

(a) Maximum w/t Ratio's Exceeded [Section B1.1(a)]: No (b) Maximum h/t Ratio's Exceeded [Section B1.2(a)]: No

File: FF100X12

PAGE P2

DATE: 8/29/2002

Section Dimensional Data

Type	Name	Gage	Height	Width	Lip	t	Weight	Coil Width
			in	in	in	in	plf	in
Panel		24	0.813	12.563	0.000	0.024	1.338	16.389

Gross Sectional Properties

Area	Ĩх	Şx	Rx	Ycg	Iy	Şy	Ry	Xcg
in2	in4	in3	in	in	in4	in3	in	in
0.393	0.015	0.022	0.198	0.097	7.701	1.098	4.425	7.012

Effective Properties

Vnx	Ix	Sx	Mnx	Mny	Įy	Sy	Pne	Pnei
kip	in4	<u>in3</u>	kip-in	kip-in	in4	in3	kip	kip/in
1.450	0.015	0.022	0.906	Ť			0.738	0.650

Torsional Properties

Xo	Ro	Beta	Cw	Jv*1000	Fy	Fu	E	G
in	in		in6	in4	ksi	ksi	ksi	ksi
-0.293	4.439	0.996	0.599	0.076	50	65	29500	11300

Shear, moment and bearing values shown are nominal values and must be modified by the appropriate factors of safety (ASD) or resistance factors (LRFD).

Factors of Safety (AS	SD)	Resistance Factors (LRFD)			
FS (Compression)	= 1.80	RF (Compression)	= 0.85		
FS (Tension)	= 1.67	RF (Tension)	= 0.95		
FS (Web Crippling)	= 1.85	RF (Web Crippling)	= 0.75		
FS (Bending)	= 1.67	RF (Bending)	= 0.90		
FS (Shear)	= 1.50	RF (Shear)	± 1.00		

John F. Butts & Associates, Inc. 2480 Vantage Drive Colorado Springs, CO 80919 (719) 598-7666

Analysis per 1996 AISI Cold-Formed Steel Manual + 1999 Supplement 1

New Tech FF100 Panel

FILE: FF100X16

SECTION DIMENSIONS

Line #1 Angle(L) = -80.000 deg	Line #21 Angle(R) = 81.000 deg
Line #1 Radius(L) = 0.060 in	Line #21 Radius(R) = 0.063 in
Line #1 Length(L) = 0.000 in	Line #22 Length(R) = 0.194 in
Line #2 Angle(L) = 80.000 deg	Line #22 Angle(R) = 70.000 deg
Line #2 Radius(L) = 0.070 in	Line #22 Radius(R) = 0.060 in
Line #2 Length(L) = 0.683 in	Line #24 Length(R) = 0.034 in
Line #3 Angle(L) = $81.000 \deg$	Line #23 Angle(R) = -86.000 deg
Line #3 Radius(L) = 0.070 in	Line #23 Radius(R) = 0.070 in
Line #3 Length(L) $=$ 0.000 in	Line #26 Length(R) = 0.134 in
Line #4 Angle(L) = -180.000 deg	Line #24 Angle(R) = -151.000 deg
Line #4 Radius(L) = 0.070 in	Line #24 Radius(R) = 0.122 in
Line #4 Length(L) \approx 0.125 in	Line #28 Length(R) = 0.447 in
Line #5 Angle(L) = 0.000 deg	Line #25 Angle(R) = 86.000 deg
Line #5 Radius(L) = 0.000 in	Line #25 Radius(R) = 0.141 in
Line #5 Length(L) = 0.000 in	Line #30 Length(R) = 0.636 in
	• • • • • • • • • • • • • • • • • • • •
Panel Bottom Width = 16.00 in	
Panel Overall Width = 16.56 in	
Panel Overall Height = 0.81 in	
-	

Alloy: ASTM A792, G50

Fy = 50.00 ksiFv = 7.35 ksi

QUALIFICATIONS PER AISI SPECIFICATIONS

(a) Maximum w/t Ratio's Exceeded [Section B1.1(a)]: No (b) Maximum h/t Ratio's Exceeded [Section B1.2(a)]: No

File: FF100X16

PAGE P4

DATE: 8/29/2002

Section	Dim	ensional	Data

Type	Name	Gage	Height	Width	Lip	t	Weight	Coil Width
			in	in	in	in	plf	in
Panel		24	0.813	16.563	0.000	0.024	1.665	20.389

Gross Sectional Properties

Area	Ιx	Sx	Rx	Ycg	Iy	Sy	Ry	Xcg
in2	in4	<u>in3</u>	in	in	in4	in3	in	in
0.489	0.016	0.022	0.181	0.080	15.531	1.724	5.634	9.007

Effective Properties

V1		Ix	Sx	Mnx	Mny	Iy	Sy	Pne	Pnei
ki		in4	in3	kip-in	kip-in	in4	in3	kip	kip/in
1.4	_	0.016	0.022	0.911		—		0.738	0.650

Torsional Properties

Xo	Ro	Beta	Cw	Jv*1000	Fy	Fu	E	G
in	in		in6	in4	ksi	ksi	ksi ksi	ksi
-0.242	5.642	0.998	1.091	0.094	50	65	29500	11300

Shear, moment and bearing values shown are nominal values and must be modified by the appropriate factors of safety (ASD) or resistance factors (LRFD).

Factors of Safety (AS	(D)	Resistance Factors (LRFD)				
FS (Compression)	= 1.80	RF (Compression)	= 0.85			
FS (Tension)	= 1.67	RF (Tension)	= 0.95			
FS (Web Crippling)	= 1.85	RF (Web Crippling)	= 0.75			
FS (Bending)	= 1.67	RF (Bending)	= 0.90			
FS (Shear)	= 1.50	RF (Shear)	= 1.00			

John F. Butts & Associates, Inc. 2480 Vantage Drive Colorado Springs, CO 80919 (719) 598-7666

Analysis per 1996 AISI Cold-Formed Steel Manual + 1999 Supplement 1

New Tech FF100 Panel

FILE: FF100X18

SECTION DIMENSIONS

Line #1 Angle(L) =	-80.000 deg	Line #21 Angle(R) = 81.000 deg
Line #1 Radius(L) =	0.060 in	Line #21 Radius(R) = 0.063 in
Line #1 Length(L) =	0.000 in	Line #22 Length(R) = 0.194 in
Line #2 Angle(L) $=$	80.000 deg	Line #22 Angle(R) = 70.000 deg
Line #2 Radius(L) =	0.070 in	Line #22 Radius(R) = 0.060 in
Line #2 Length(L) $=$	0.683 in	Line #24 Length(R) = 0.034 in
Line #3 Angle(L) =	81.000 deg	Line #23 Angle(R) = -86.000 deg
Line #3 Radius(L) =	0.070 in	Line #23 Radius(R) = 0.070 in
Line #3 Length(L) $=$	0.000 in	Line #26 Length(R) = 0.134 in
Line #4 Angle(L) $=$	-180.000 deg	Line #24 Angle(R) = -151.000 deg
Line #4 Radius(L) =	0.070 in	Line #24 Radius(R) = 0.122 in
Line #4 Length(L) =	0.125 in	Line #28 Length(R) = 0.447 in
Line #5 Angle(L) =	0.000 deg	Line #25 Angle(R) = 86.000 deg
Line #5 Radius(L) =	0.000 in	Line #25 Radius(R) = 0.141 in
Line #5 Length(L) =	0.000 in	Line #30 Length(R) = 0.636 in
		- · · ·
Panel Bottom Width	= 18.00 in	
Panel Overall Width	= 18.56 in	

Panel Bottom Width = 18.00 in Panel Overall Width = 18.56 in Panel Overall Height = 0.81 in

Alloy: ASTM A792, G50

Fy $\approx 50.00 \text{ ksi}$ Fv = 5.99 ksi

QUALIFICATIONS PER AISI SPECIFICATIONS

(a) Maximum w/t Ratio's Exceeded [Section B1.1(a)]: No (b) Maximum h/t Ratio's Exceeded [Section B1.2(a)]: No

File : FF100X18 PAGE P6

DATE: 8/29/2002

Section Dimensional Data

Type	Name	Gage	Height	Width	Lip	t	Weight	Coil Width
			in	in	in	in	plf	in
Panel		24	0.813	18.563	0.000	0.024	1.828	22.389

Gross Sectional Properties

Атеа	Įх	Sx	Rx	Ycg	Iy	Sy	Ry	Xcg
in2	in4	in3	in	in	in4	in3	in	in
0.537	0.016	0.022	0.174	0.074	20.866	2.085	6.231	10.005

Effective Properties

Vnx	Ix	Sx	Mnx	Mny	Iy	Sy	Pne	Pnei
kip	in4	in3	kip-in	kip-in	in4	in3	kip	kip/in
1.450	0.016	0.022	0.910				0.738	0.650

Torsional Properties

X	5	Ro	Beta	Cw	Jv*1000	Fy	Fu	E	G
ir	ı	in		in6	in4	ksi	ksi	ksi	ksi
-0.2	22	6.238	0.999	1.395	0.103	50	65	29500	11300

Shear, moment and bearing values shown are nominal values and must be modified by the appropriate factors of safety (ASD) or resistance factors (LRFD).

Factors of Safety (AS	(D)	Resistance Factors (LRFD)			
FS (Compression)	= 1.80	RF (Compression)	= 0.85		
FS (Tension)	= 1.67	RF (Tension)	= 0.95		
FS (Web Crippling)	= 1.85	RF (Web Crippling)	= 0.75		
FS (Bending)	= 1.67	RF (Bending)	= 0.90		
ES (Shear)	= 1.50	RF (Shear)	= 1.00		

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File : FF100X12

Width | 12.56 in

Alloy | ASTM A792, G50 (Fy =50 ksi)

Gauge 24 (0.024 in)

Allowable Strength Design (ASD) Wind Load Factor = 1.00 Allowable Uniform Load (psf)

Page: 1

Date: 8/29/2002

<u>Span</u>	Deflection	n .	Span Length (Feet)							
		2.00	2.25	2.50	2.75	3.00	3.25	3.50	3.75	4.00
1	L/180	82	65	52	43	36	30	26	22	19
	L/240	82	65	52	43	35	27	22	18	15
	L/360	78	55	40	30	23	18	15	12	10
2	L/180	82	65	52	43	36	30	26	22	20
	L/240	82	65	52	43	36	30	26	22	20
	L/360	82	65	52	42	33	26	21	17	14
3	L/180	95	75	61	50	42	35	30	26	23
	L/240	95	7 5	61	50	42	3 5	30	26	23
	L/360	95	75	61	50	42	34	27	22	18

1. Formula's used in Load Tables for FLEXURE and DEFLECTION are:

One Span - Mp = .125wl^2, Mn = .125wl^2, $x = .0130wl^4/EI$ Two Span - Mp = .125wl^2, Mn = .096wl^2, $x = .0092wl^4/EI$ Three Span - Mp = .080wl^2, Mn = .107wl^2, $x = .0069wl^4/EI$ Modulas of Elasticity (E) = 29500 ksi

- 2. Allowable uniform loads are determined per the following:
 - a) Allowable Shear Stress (Fv) [AISI, C3.2] b) Combined Bending and Shear [AISI, C3.3]
 - c) Combined Bending & Web Crippling [AISI C3.5]
- 3. Factors of Safety used to determine uniform loads:

FS (Bending) = 1.67 FS (Shear) = 1.50 FS (Web Crippling) = 1.85

- 4. Allowance has been made for member Dead Weight.
- 5. Minimum panel support bearing length = 2.00 in
- 6. Concentrated load = 250 lbs at mid-span, load width = 4 in
 Simple Span : Maximum Span = 1.361 ft (L/180)
 Two Span : Maximum Span = 1.560 ft (L/180)
 Three Span + : Maximum Span = 1.648 ft (L/180)

File: FF100X12

Width | 12.56 in

Alloy | ASTM A792, G50 (Fy =50 ksi)

Gauge | 24 (0.024 in)

Allowable Strength Design (ASD) Wind Load Factor = 1.00 Allowable Uniform Load (psf)

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Date: 8/29/2002

<u>Span</u>	Deflection	1	Span Length (Feet)							
		4.25	4.50	4.75	5.00	5.25	5.50	5.75	6.00	6.25
1	L/180	16	14	12	10	9	7	7	6	5
	L/240	12	10	9	7	6	6	5	4	4
	L/360	8	7	6	5	4	4	3	3	3
2	L/180	17	15	14	12	11	10	9	8	7
	L/240	17	15	12	11	9	8	7	6	5
	L/360	11	10	8	7	6	5	5	4	4
3	L/180	20	18	16	14	13	12	11	10	9
	L/240	20	18	16	14	12	11	9	8	7
	L/360	15	13	11	9 _.	8	7	6	5	5

1. Formula's used in Load Tables for FLEXURE and DEFLECTION are:

One Span $-Mp = .125wl^2$, $Mn = .125wl^2$, $x = .0130wl^4/EI$ $-Mp = .125wl^2$, $Mn = .096wl^2$, $x = .0092wl^4/EI$ Two Span $-Mp = .080wl^2$, $Mn = .107wl^2$, $x = .0069wl^4/EI$

Modulas of Elasticity (E) = 29500 ksi

- 2. Allowable uniform loads are determined per the following:
 - a) Allowable Shear Stress (Fv)

[AISI, C3.2]

b) Combined Bending and Shear

[AJSI, C3.3]

c) Combined Bending & Web Crippling

[AISI C3.5]

3. Factors of Safety used to determine uniform loads:

FS (Bending) = 1.67

= 1.50

FS (Shear)

FS (Web Crippling) = 1.85

- 4. Allowance has been made for member Dead Weight.
- 5. Minimum panel support bearing length = 2.00 in
- 6. Concentrated load = 250 lbs at mid-span, load width = 4 in

Simple Span : Maximum Span = 1.361 ft (L/180) Two Span : Maximum Span = 1.560 ft (L/180)

Three Span + : Maximum Span = 1.648 ft (L/180)

File : FF100X16

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Date: 8/29/2002

Width | 16.56 in

Alloy | ASTM A792, G50 (Fy =50 ksi)

Gauge | 24 (0.024 in)

Allowable Strength Design (ASD) Wind Load Factor = 1.00 Allowable Uniform Load (psf)

<u>Span</u>	Deflection	<u>i</u> _			Span L	ength (Feet	<u>()</u>			
		2.00	2.25	2.50	2.75	3.00	3.25	3.50	3.75	4.00
1	L/180 L/240	63 63	50 50	40 40	33	27 27	23 22	20 17	17 14	15 12
	L/360	62	43	32	24	18	14	12	9	8
2	L/180 L/240	63 63	50 50	40 40	33 33	27 27	23 23	20 20	17 17	15 15
	1./360	63	50 — - 57	40	33	26	20	16	13	11
3	L/180 L/240 L/360	73 73 73	57 57 57	46 46 46	38 38 38	32 32 32	27 27 27	23 23 22	20 20 18	17 17 15

1. Formula's used in Load Tables for FLEXURE and DEFLECTION are:

One Span $-Mp = .125wl^2$, $Mn = .125wl^2$, $x = .0130wl^4/EI$ Two Span $-Mp = .125wl^2$, $Mn = .096wl^2$, $x = .0092wl^4/EI$ Three Span $-Mp = .080wl^2$, $Mn = .107wl^2$, $x = .0069wl^4/EI$

Modulas of Elasticity (E) = 29500 ksi

2. Allowable uniform loads are determined per the following:

a) Allowable Shear Stress (Fv)

[AISI, C3.2]

b) Combined Bending and Shear

[AISI, C3.3]

c) Combined Bending & Web Crippling

[AISI, C3.5]

3. Factors of Safety used to determine uniform loads:

FS (Bending)

= 1.67

FS (Shear)

= 1.50

FS (Web Crippling) = 1.85

- 4. Allowance has been made for member Dead Weight.
- 5. Minimum panel support bearing length = 2.00 in
- 6. Concentrated load = 250 lbs at mid-span, load width = 4 in

Simple Span : Maximum Span = 1.376 ft (L/180) Two Span : Maximum Span = 1.577 ft (L/180) Three Span + : Maximum Span = 1.667 ft (L/180)

File : FF100X18

Width | 18.56 in

Alloy | ASTM A792, G50 (Fy =50 ksi)

Gauge | 24 (0.024 in)

Allowable Strength Design (ASD) Wind Load Factor = 1.00 Allowable Uniform Load (psf)

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<u>Span</u>	Deflection	9			Span L	ength (Feet	t)			
		2.00	2.25	2.50	2.75	3.00	3.25	3.50	3.75	4.00
1	L/180	56	44	.36	29	24	21	18	15	13
	L/240	56	4 4	36	29	24	20	16	13	10
	L/360	5 6	39	29	22	17	13	10	8	7
2	L/180	56	44	36	29	24	21	18	15	13
	L/240	56	44	36	29	24	21	18	15	13
	L/360	56	44	36	29	23	18	15	12	10
3	L/180	65	51	41	34	28	24	21	18	16
	L/240	65	51	41	34	28	24	21	18	16
	L/360	65	51	41	34	28	24	20	16	13

1. Formula's used in Load Tables for FLEXURE and DEFLECTION are:

One Span $-Mp = .125wi^2$, $Mn = .125wi^2$, $x = .0130wi^4/EI$ $-Mp = .125wl^2$, $Mn = .096wl^2$, $x = .0092wl^4/EI$ Two Span Three Span $-Mp = .080wl^2$, $Mn = .107wl^2$, $x = .0069wl^4/EI$

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- 4. Allowance has been made for member Dead Weight.
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- 6. Concentrated load = 250 lbs at mid-span, load width = 4 in

Simple Span : Maximum Span = 1.379 ft (L/180)

Two Span

: Maximum Span = 1.581 ft (L/180)

Three Span + : Maximum Span = 1.672 ft (L/180)

	$f_{i,j} + \dots + f_{i,j}$
	No see the
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